



<b>Company Name</b>	IIT Bombay	<b>Project Title</b>	Connection Design Examples
<b>Group/Team Name</b>	Osdag	<b>Subtitle</b>	Seated angle shear connection
<b>Designer</b>	Engineer #1	<b>Job Number</b>	1.1.4.2.2
<b>Date</b>	20 /06 /2018	<b>Client</b>	Yogesh D Pisal, Aker Powergas Ltd, Pune

### Design Conclusion

<b>Seated Angle</b>	<b>Fail</b>
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### Seated Angle

### Connection Properties

#### Connection

Connection Title	Seated Angle
Connection Type	Shear Connection

#### Connection Category

Connectivity	Column web-Beam flange
Beam Connection	Bolted
Column Connection	Bolted

#### Loading (Factored Load)

Shear Force (kN)	140.0
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#### Components

<b>Column Section</b>	HB 200
Material	Fe 410
Hole	Over-sized
<b>Beam Section</b>	WPB 140x140x24.7
Material	Fe 410
Hole	Over-sized
<b>Seated Angle Section</b>	150 150 X 15
Material	Fe 410
Hole	Over-sized
<b>Top Angle Section</b>	90 90 x 10
Material	Fe 410
Hole	Over-sized

#### Bolts

Type	Bearing Bolt
Grade	5.8
Diameter (mm)	12
Bolts - Required	8
Bolts - Provided	8
Rows	2
Columns	4

Gauge (mm)	30
Pitch (mm)	31.0
End Distance (mm)	70
Edge Distance (mm)	25
<b>Assembly</b>	
Column-Beam Clearance (mm)	5.0



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**Design Preferences****Bolt**

Hole Type	Over-sized Hole
Material Grade Fu (MPa) (overwrite)	800

**Detailing**

Type of Edge	Rolled, machine-flame cut, sawn and planed
Minimum Edge Distance check multiplier	1.5 * bolt_hole_diameter
Are members exposed to corrosive influences?	No
Gap between Beam and Column (mm)	5.0

**Design**

Design Method	Limit State Design
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Design Check			
Check	Required	Provided	Remark
<b>Bolt Checks</b>			
<b>Bolt shear capacity (kN)</b>	$V_{dsb} = \text{bolt\_fu}^*$ $(\pi * 0.78 / 4) * \text{bolt\_diameter}^2 / (\sqrt{3}) / \gamma_{mb}$ [cl. 10.3.3]	$V_{dsb} = 500^*$ $(0.6126) * 12^2 / (\sqrt{3}) / 1.25 / 1000$ $= 19.5$	
<b>Bolt bearing capacity (kN)</b>	$V_{dpb}$ [Cl. 10.3.4]	$V_{dpb} =$ $2.5 * 0.417 * 12 * 5.5 * 410 / 1.25 / 1000$ $= 36.9 \text{ kN}$	
<b>Bolt capacity (kN)</b>	$\min(\text{bolt\_shear\_capacity},$ $\text{bolt\_bearing\_capacity})$	$\min(19.5, 36.9) = 19.5$	
<b>No. of bolts</b>	$140.0 / 19.5 = 8.0$	8	<b>Pass</b>
<b>No. of columns</b>		4	
<b>No. of row(s)</b>	$\leq 2$	2	
<b>Bolt pitch (mm)</b>	$\geq 2.5 * 12 = 30,$ $\leq \min(32 * 9.0, 300) = 288.0$ [cl. 10.2.2]	31.0	<b>Pass</b>
<b>Bolt gauge (mm)</b>	$\geq 2.5 * 12 = 30,$ $\leq \min(32 * 9.0, 300) = 288.0$ [cl. 10.2.2]	30	<b>Pass</b>
<b>End distance (mm)</b>	$\geq 1.5 * 15 = 23$	70	<b>Pass</b>
<b>Edge distance (mm)</b>	$\geq 1.5 * 15 = 23$ [cl. 10.2.4.2] $\leq 12 * 9.0 \sqrt{250/250} = 108.0$ [Cl 10.2.4.3]	25	<b>Pass</b>
<b>Seated Angle 150 150 X 15</b>			
<b>Length (mm)</b>	$= \min(140.0, 200.0 - 2 * 9.0 - 2 * 9.0 - 18.0)$	140	
<b>Outstanding leg length (mm)</b>	[Cl. 8.7.4] $= (140.0 * 1000 * 1.1 / (250 * 5.5)) + 5.0$	150	<b>Pass</b>
<b>Shear</b>	$V_{dp} \geq V$		

<b>capacity of outstanding leg (kN)</b>	$V_{dp} \geq 140.0\text{kN}$ [Cl. 8.4.1]	$= (140 \cdot 15.0) \cdot 250 / (\sqrt{3} \cdot 1.1)$ $= 333.4$	<b>Pass</b>
<b>Moment capacity of outstanding leg (kN-mm)</b>	As $V \leq 0.6 V_d$ , [Cl 8.2.1.2] is applicable $M_d \geq \text{Moment at root of angle}$ $M_d \geq 3983.6$	$M_d = \min(\beta_b Z_e f_y / \gamma_{m0}, 1.5 Z_e f_y / \gamma_{m0})$ $= \min(1.0 \cdot 140 \cdot (15.0^2 / 6) \cdot 250 / 1.1, 1.5 \cdot 140 \cdot (15.0^2 / 6) \cdot 250 / 1.1)$ $= 1193.2$	<b>Fail</b>
<b>Top Angle</b>			
<b>Section</b>	Recommended size (based on stability only): 35 35 X 4	User selected size: 90 90 x 10	
<b>End distance (mm)</b>	$\geq 1.5 \cdot \text{bolt\_hole\_diameter}$ [cl. 10.2.4.2] $\geq 1.5 \cdot 15 = 23$	on leg connected to Beam: 35 on leg connected to Column: 35	<b>Pass</b>



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**Views**



Created with

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<b>Additional Comments</b>	
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