



<b>Company Name</b>	<b>IIT Bombay</b>	<b>Project Title</b>	<b>Connection Design Examples</b>
<b>Group/Team Name</b>	<b>Osdag</b>	<b>Subtitle</b>	<b>Seated angle shear connection</b>
<b>Designer</b>	<b>Engineer #1</b>	<b>Job Number</b>	<b>1.1.4.1.1</b>
<b>Date</b>	<b>20 /06 /2018</b>	<b>Client</b>	<b>Somnath Mukherjee</b>

**Design Conclusion**

<b>Seated Angle</b>	<b>Pass</b>
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**Seated Angle**

**Connection Properties**

**Connection**

Connection Title	Seated Angle
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Connection Type	Shear Connection
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**Connection Category**

Connectivity	Column flange-Beam flange
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Beam Connection	Bolted
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Column Connection	Bolted
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**Loading (Factored Load)**

Shear Force (kN)	100.0
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**Components**

<b>Column Section</b>	UC 203 x 203 x 86
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Material	Fe 410
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Hole	Standard
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<b>Beam Section</b>	MB 300
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Material	Fe 410
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Hole	Standard
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<b>Seated Angle Section</b>	150 150 X 15
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Material	Fe 410
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Hole	Standard
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<b>Top Angle Section</b>	150 150 X 10
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Material	Fe 410
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Hole	Standard
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**Bolts**

Type	Friction Grip Bolt
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Grade	10.9
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Diameter (mm)	20
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Bolts - Required	2
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Bolts - Provided	2
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Rows	1
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Columns	2
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Gauge (mm)	60.0
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Pitch (mm)	0.0
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End Distance (mm)	85
Edge Distance (mm)	40
<b>Assembly</b>	
Column-Beam Clearance (mm)	10.0



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**Design Preferences**

**Bolt**

Hole Type	Standard Hole
Material Grade Fu (MPa) (overwrite)	940
Slip Factor	0.55

**Detailing**

Type of Edge	Sheared or hand flame cut
Minimum Edge Distance check multiplier	1.7 * bolt_hole_diameter
Are members exposed to corrosive influences?	No
Gap between Beam and Column (mm)	10.0

**Design**

Design Method	Limit State Design
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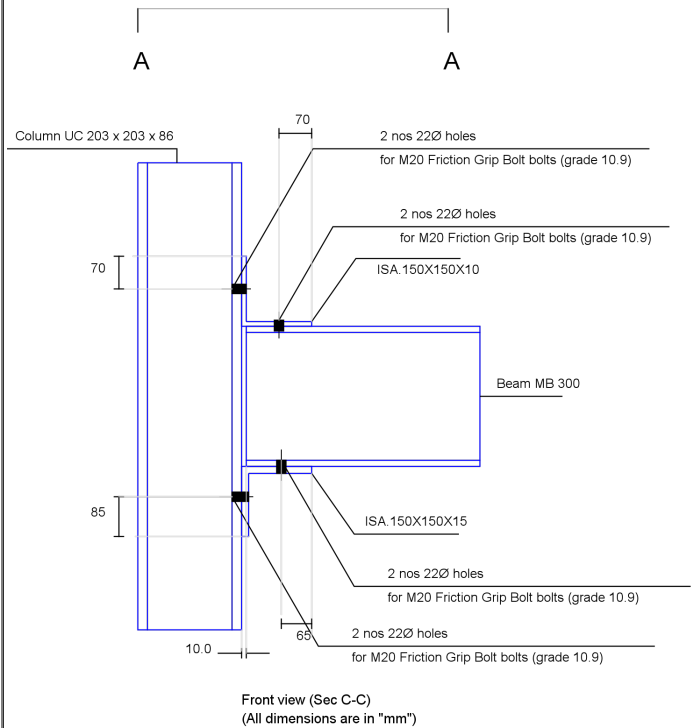
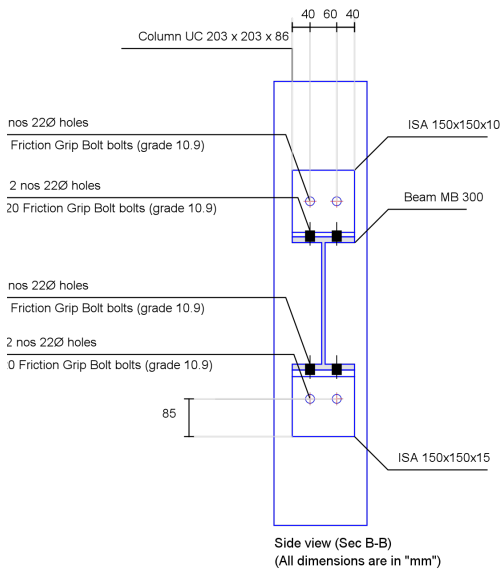
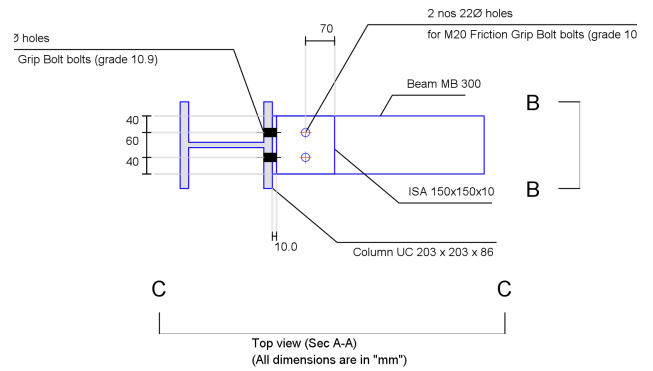
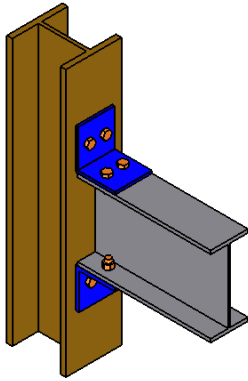
<b>Design Check</b>			
<b>Check</b>	<b>Required</b>	<b>Provided</b>	<b>Remark</b>
<b>Bolt Checks</b>			
<b>Bolt shear capacity (kN)</b>	Friction Grip Bolt bolt shear capacity: [cl. 10.3.3]	$V_{dsf} = (0.55)*(1)*(1.0)*(0.6126*20^2) * (0.70*1000)/1.25/1000 = 75.46$	
<b>Bolt bearing capacity (kN)</b>	$V_{dpb}$ : [Cl. 10.3.4]	N/A	
<b>Bolt capacity (kN)</b>	min (bolt_shear_capacity, bolt_bearing_capacity)	min (75.46, 0.0) = 75.46	
<b>No. of bolts</b>	$100.0/75.46 = 2.0$	2	<b>Pass</b>
<b>No. of columns</b>		2	
<b>No. of row(s)</b>	$\leq 2$	1	
<b>Bolt pitch (mm)</b>	N/A	N/A	
<b>Bolt gauge (mm)</b>	$\geq 2.5*20 = 50,$ $\leq \min(32*15.0, 300) = 300.0$ [cl. 10.2.2]	60.0	<b>Pass</b>
<b>End distance (mm)</b>	$\geq 1.7*22 = 38$	85	<b>Pass</b>
<b>Edge distance (mm)</b>	$\geq 1.7*22 = 38$ [cl. 10.2.4.2] $\leq 12*15.0\sqrt{250/250} = 180.0$ [Cl 10.2.4.3]	40	<b>Pass</b>
<b>Seated Angle 150 150 X 15</b>			
<b>Length (mm)</b>	$= \min(140.0, 209.1)$	140	
<b>Outstanding leg length (mm)</b>	[Cl. 8.7.4] $= (100.0*1000*1.1/(250*7.7)) + 10.0$	150	<b>Pass</b>
<b>Shear capacity of outstanding leg (kN)</b>	$V_{dp} \geq V$ $V_{dp} \geq 100.0\text{kN}$ [Cl. 8.4.1]	$= (140*15.0)*250/(\sqrt{3}*1.1)$ $= 333.4$	<b>Pass</b>

<b>Moment capacity of outstanding leg (kN-mm)</b>	As $V \leq 0.6 V_d$ , [Cl 8.2.1.2] is applicable $M_d \geq$ Moment at root of angle $M_d \geq 136.1$	$M_d = \min(\beta_b Z_e f_y / \gamma_{m0}, 1.5 Z_e f_y / \gamma_{m0})$ $= \min(1.0 * 140 * (15.0^2 / 6) * 250 / 1.1, 1.5 * 140 * (15.0^2 / 6) * 250 / 1.1)$ $= 1193.2$	<b>Pass</b>
<b>Top Angle</b>			
<b>Section</b>	Recommended size (based on stability only): 80 80 X 8	User selected size: 150 150 X 10	
<b>End distance (mm)</b>	$\geq 1.7 * \text{bolt\_hole\_diameter}$ [cl. 10.2.4.2] $\geq 1.7 * 22 = 38$	on leg connected to Beam: 70 on leg connected to Column: 70	<b>Pass</b>



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## Views





IIT Bombay



Created with

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<b>Additional Comments</b>	This is a sample design report generated in Osdag!
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