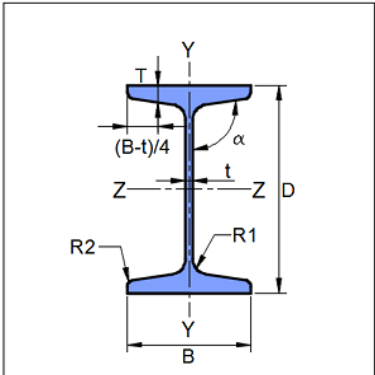




Company Name	IIT Bombay	Project Title	Base Plate
Group/Team Name	Osdag	Subtitle	Welded Base Plate
Designer	Engineer#1	Job Number	1.3.1.1
Date	04 /02 /2021	Client	Prof. S R Satish Kumar, IIT Madras

1 Input Parameters

Main Module	Moment Connection			
Module	Base Plate Connection			
Connectivity	Welded Column Base			
End Condition	Pinned			
Axial Compression (kN)	1100.0			
Axial Tension/Uplift (kN)	0.0			
Shear Force (kN)				
- Along major axis (z-z)	0.0			
- Along minor axis (y-y)	0.0			
Bending Moment (kNm)				
- Major axis (M_{z-z})	0.0			
- Minor axis (M_{y-y})	0.0			
Column Section - Mechanical Properties				
	Column Section		HB 450	
	Material		E 250 (Fe 410 W)A	
	Ultimate Strength, F_u (MPa)		410.0	
	Yield Strength, F_y (MPa)		250.0	
	Mass, m (kg/m)	87.22	I_z (cm ⁴)	39200.0
	Area, A (cm ²)	111.0	I_y (cm ⁴)	2980.0
	None	None	r_z (cm)	18.7
	D (mm)	450.0	r_y (cm)	5.18
	B (mm)	250.0	Z_z (cm ³)	1740.0
	T (mm)	13.7	Z_y (cm ³)	238.0
	t (mm)	9.8	Z_{pz} (cm ³)	1950.0
	Flange Slope	94	Z_{py} (cm ³)	394.0
	R_1 (mm)	15.0		
	R_2 (mm)	7.5		
Base Plate - Design Preference				
Material		E 250 (Fe 410 W)A		
Ultimate Strength, F_u (MPa)		410		
Yield Strength, F_y (MPa)		250		



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Stiffener/Shear Key - Design Preference	
Material	E 250 (Fe 410 W)A
Anchor Bolt Outside Column Flange - Input and Design Preference	
Diameter (mm)	['M20', 'M24', 'M30']
Property Class	['8.8', '9.8']
Anchor Bolt Type	End Plate Type
Anchor Bolt Galvanized?	Yes
Designation	M20X348.5 IS5624 GALV
Hole Type	Over-sized
Total Length (mm)	348.5
Material Grade, F_u (MPa)	900.0
Anchor Bolt Inside Column Flange - Input and Design Preference	
Diameter (mm)	N/A
Property Class	N/A
Anchor Bolt Type	N/A
Anchor Bolt Galvanized?	N/A
Designation	N/A
Hole Type	N/A
Total Length (mm)	N/A
Material Grade, F_u (MPa)	N/A
Friction Coefficient (between concrete and anchor bolt)	0.3
Weld - Design Preference	
Type of Weld Fabrication	Shop Weld
Material Grade Overwrite, F_u (MPa)	510.0
Detailing - Design Preference	
Edge Preparation Method	b - Rolled, machine-flame cut, sawn and planed
Are the Members Exposed to Corrosive Influences?	Yes
Design - Design Preference	
Design Method	Limit State Design
Base Plate Analysis	Effective Area Method



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2 Design Checks

Design Status	Pass
---------------	------

2.1 Design Parameters

Check	Required	Provided	Remarks
Bearing Strength of Concrete (N/mm ²)		$\sigma_{br} = 0.45f_{ck}$ $= 0.45 \times 25$ $= 11.25$ [Ref. IS 456:2000, Cl.34.4]	OK
Grout Thickness (mm)		$t_g = 50$	OK
Epsilon - stiffener plate		$\epsilon_{st} = \sqrt{\frac{250}{f_{yst}}}$ $= \sqrt{\frac{250}{250}}$ $= 1.0$ [Ref. IS 800:2007, Table2]	OK

2.2 Load Consideration

Check	Required	Provided	Remarks
Axial Compression (kN)	$P_x = 1100.0$	$P_u = \max(P_x, 0.3P_d)$, but, $\leq P_d$ $= \max(1100.0, 0.3 \times 2522.73)$ $= \max(1100.0, 756.82)$ ≤ 2522.73 $= 1100.0$ [Ref. IS 800:2007, Cl.10.7] Note: P_d is the design axial capacity of the column	Pass
Shear Force - along major (z-z) axis (kN)	$V_d = 326.06$	$V_1 = 0.0$	
Shear Force - along minor (y-y) axis (kN)	$V_d = 326.06$	$V_2 = 0.0$	



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Check	Required	Provided	Remarks
Interaction Ratio	I.R. < 1.0	$\begin{aligned} \text{I.R. axial} &= P_x/P_d \\ &= 1100.0/2522.73 \\ &= 0.44 \\ \\ \text{I.R. moment} &= M_z/M_{dz} \\ &= 0.0/0.0 \\ &= 0.0 \\ \\ \text{I.R. sum} &= \text{I.R. axial} + \text{I.R. moment} \\ &= 0.44 + 0.0 \\ &= 0.44 \end{aligned}$	Pass

2.3 Plate Washer and Nut Details - Anchor Bolt Outside Column Flange

Check	Required	Provided	Remarks
Plate Washer Size (mm)		Square – 45X45 [Ref. IS 6649:1985, Table 2]	Pass
Plate Washer Thickness (mm)		$t_w = 8.5$ [Ref. IS 6649:1985, Table 2]	Pass
Plate Washer Hole Diameter (mm)		$d_h = 22$ [Ref. IS 6649:1985, Table 2]	Pass
Nut (hexagon) Thickness (mm)		$t_n = 18.0$ [Ref. IS 1364-3:2002, Table 1]	Pass
End Plate Size (mm)		Square - 90 X 90	Pass
End Plate Thickness (mm)		14	Pass



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2.4 Anchor Bolt Summary - Outside Column Flange

Check	Required	Provided	Remarks
Diameter (mm)		20	Pass
Number of Bolts		$n_{out} = 4$	Pass
Property Class		9.8	Pass

2.5 Anchor Bolt Summary - Inside Column Flange

Check	Required	Provided	Remarks
Diameter (mm)	0	N/A	N/A
Number of Bolts	0	$n_{in} = 0$	N/A
Property Class	N/A	N/A	N/A

2.6 Detailing Checks - Outside Column Flange

Check	Required	Provided	Remarks
Min. End Distance (mm)	$e_{min} = 1.5d_0$ $= 1.5 \times 24.0$ $= 36.0$ [Ref. IS 800:2007, Cl.10.2.4.2]	55	Pass
Max. End Distance (mm)	$e_{max} = 40 + 4t$ Where, $t = \min(14, 14)$ $= 40 + (4 \times 14)$ $e_{max} = 96.0$ [Ref. IS 800:2007, Cl.10.2.4.3]	55	Pass
Min. Edge Distance (mm)	$e'_{min} = 1.5d_0$ $= 1.5 \times 24.0$ $= 36.0$ [Ref. IS 800:2007, Cl.10.2.4.2]	55	Pass



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Check	Required	Provided	Remarks
Max. Edge Distance (mm)	$e'_{\max} = 40 + 4t$ Where, $t = \min(14, 14)$ $= 40 + (4 \times 14)$ $e'_{\max} = 96.0$ [Ref. IS 800:2007, Cl.10.2.4.3]	55	Pass
Min. Pitch Distance (mm)	N/A	0.0	N/A
Max. Pitch Distance (mm)	N/A	0.0	N/A

2.7 Base Plate Dimension (L X W)

Check	Required	Provided	Remarks
Length (mm)	$L = D + 2(e + e')$ $= 450.0 + 2 \times (55 + 55)$ $= 670.0$ [Ref. based on detailing requirement]	670.0	Pass
Width (mm)	$W = (0.85B) + 2(e' + e')$ $= (0.85 \times 250.0) + 2 \times (55 + 55)$ $= 432.5$ [Ref. based on detailing requirement]	470.0	Pass

2.8 Base Plate Analysis

Check	Required	Provided	Remarks
Min. Area Required (mm^2)	$A_{\text{reqmin}} = \frac{P_u}{\sigma_{br}}$ $= \frac{1100.0 \times 10^3}{11.25}$ $= 97.78 \times 10^3$	$A_{\text{provided}} = L \times W$ $= 670.0 \times 470.0$ $= 314.9 \times 10^3$	Pass



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Designer	Engineer#1	Job Number	1.3.1.1
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Check	Required	Provided	Remarks
Effective Bearing Area (mm ²)	$A_{\text{breff}} = (D + 2c)(B + 2c) - \left[(D - 2(T + c))(B - t) \right]$ $= (450.0 + 2c)(250.0 + 2c) - \left[(450.0 - 2 \times (13.7 + c))(250.0 - 9.8) \right]$ <p>Note: c is the projection beyond the face of the column.</p> <p>[Reference: Design of Steel Structures, by N.Subramanian, (2019 edition)]</p>		OK
Projection (mm)	$A_{\text{breff}} = A_{\text{req min}}$ $= 97.78 \times 10^3$ <p>Therefore, $(450.0 + 2c)(250.0 + 2c) - \left[(450.0 - 2(13.7 + c))(250.0 - 9.8) \right]$</p> $= 97.78 \times 10^3$ $c = 10.85$ <p>projection = max(c, e)</p> $= \max(10.85, 55)$ $= 55$ <p>[Reference: Design of Steel Structures, by N.Subramanian, (2019 edition)]</p>	55	Pass
Actual Bearing Stress (N/mm ²)	11.25	$\sigma_{\text{bractual}} = \frac{P_u}{A_{\text{provided}}}$ $= \frac{1100.0 \times 10^3}{314.9 \times 10^3}$ $= 3.49$	Pass



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Check	Required	Provided	Remarks
Thickness of Base Plate (mm)	$(T, t) < t_p \leq 120$ $(13.7, 9.8) < t_p \leq 120$	$t_p = c \left[\frac{2.5 \sigma_{bractual} \gamma_{m0}}{f_{y_{plate}}} \right]^{0.5}$ $= 55 \times \left[\frac{2.5 \times 3.49 \times 1.1}{250} \right]^{0.5}$ $= 10.78$ $= 14$ <p>[Ref. IS 800:2007, Cl.7.4.3.1]</p>	Pass

2.9 Anchor Bolt Design - Outside Column Flange

Check	Required	Provided	Remarks
Shear Capacity (kN)		$V_{dsb} = \frac{f_{ub} n_n A_{nb}}{\sqrt{3} \gamma_{mb}}$ $= \frac{900.0 \times 1 \times 245}{1000 \times \sqrt{3} \times 1.25}$ $= 101.84$ <p>[Ref. IS 800:2007, Cl.10.3.3]</p>	OK
Kb		$k_b = \min \left(\frac{e}{3d_0}, \frac{f_{ub}}{f_u}, 1.0 \right)$ $= \min \left(\frac{55}{3 \times 24.0}, \frac{900.0}{410.0}, 1.0 \right)$ $= \min(0.76, 2.2, 1.0)$ $= 0.76$ <p>[Ref. IS 800:2007, Cl.10.3.4]</p>	OK



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Check	Required	Provided	Remarks
Bearing Capacity (kN)		$V_{dpb} = \frac{2.5k_b d t f_u}{\gamma_{mb}}$ $= \frac{2.5 \times 0.76 \times 20 \times 14 \times 410}{1000 \times 1.25}$ $= 174.5$ $= 0.7 \times 174.5$ $= 122.15$ <p>Note: The bearing capacity is reduced since the hole type is Over-sized or Short-slotted.</p> <p>[Ref. IS 800:2007, Cl.10.3.4]</p>	OK
Bolt Capacity (kN)		$V_{db} = \min (V_{dsb}, V_{dpb})$ $= \min (101.84, 122.15)$ $= 101.84$ <p>[Ref. IS 800:2007, Cl.10.3.2]</p>	OK
Tension Demand - per anchor bolt (kN)	$T_b = \frac{P_t}{n_{out}/2}$ $= \frac{0}{4/2}$ $= \frac{0}{2}$ $= 0.0$	$T_{db} = 0.90 f_{ub} A_n / \gamma_{mb}$ $< f_{yb} A_{sb} (\gamma_{mb} / \gamma_{m0})$ $= \min \left(0.90 \times 900.0 \times 245 / 1.25, \right.$ $\left. 720.0 \times 314 \times (1.25/1.1) \right)$ $= \min(158.76, 256.91)$ $= 158.76$ <p>[Ref. IS 800:2007, Cl.10.3.5]</p>	
Anchor Length - above concrete footing (mm)		$l_1 = t_g + t_p + t_w + t_n + 20$ $= 50 + 14 + 8.5 + 18.0 + 20$ $= 110.5$	Pass
Anchor Length - below concrete footing (mm)		$l_2 = 238.0$ <p>[Reference: IS 5624:1993, Table 1.]</p>	Pass



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Check	Required	Provided	Remarks
Anchor Length - total (mm)	$200 \leq l_a \leq 800$ [Reference: IS 5624:1993, Table 1]	$l_a = l_1 + l_2$ $= 110.5 + 238.0$ $= 348.5$	Pass

2.10 Stiffener Design - Across Column Web

Check	Required	Provided	Remarks
Length of Stiffener (mm)		$L_{staw} = 110.0$	Pass
Height of Stiffener (mm)		$H_{staw} = L_{staw} + 50$ $= 110.0 + 50$ $= 160.0$	Pass
Thickness of Stiffener (mm)	$t_{staw} = \left(\frac{L_{staw}}{13.6 \times \epsilon_{st}} \right) \geq t$ $= \max \left(\left(\frac{110.0}{13.6 \times 1.0} \right), 9.8 \right)$ $= \max(8.09, 9.8)$ [Ref. IS 800:2007, Table 2.]	10	Pass
Weld Size (mm)	3	6	Pass

2.11 Weld Design - Column to Base Plate Connection

Check	Required	Provided	Remarks
Weld Strength (N/mm ²)	$f_{uw} = \min(f_w, f_u)$ $= \min(510.0, 410.0)$ [Ref. IS 800:2007, Cl.10.5.7.1.1]	$f_{uw} = 410.0$	Pass
Total Weld Length - at flange (mm)		870	Pass
Total Weld Length - at web (mm)		745	Pass
Weld Size (mm)	5	8	Pass



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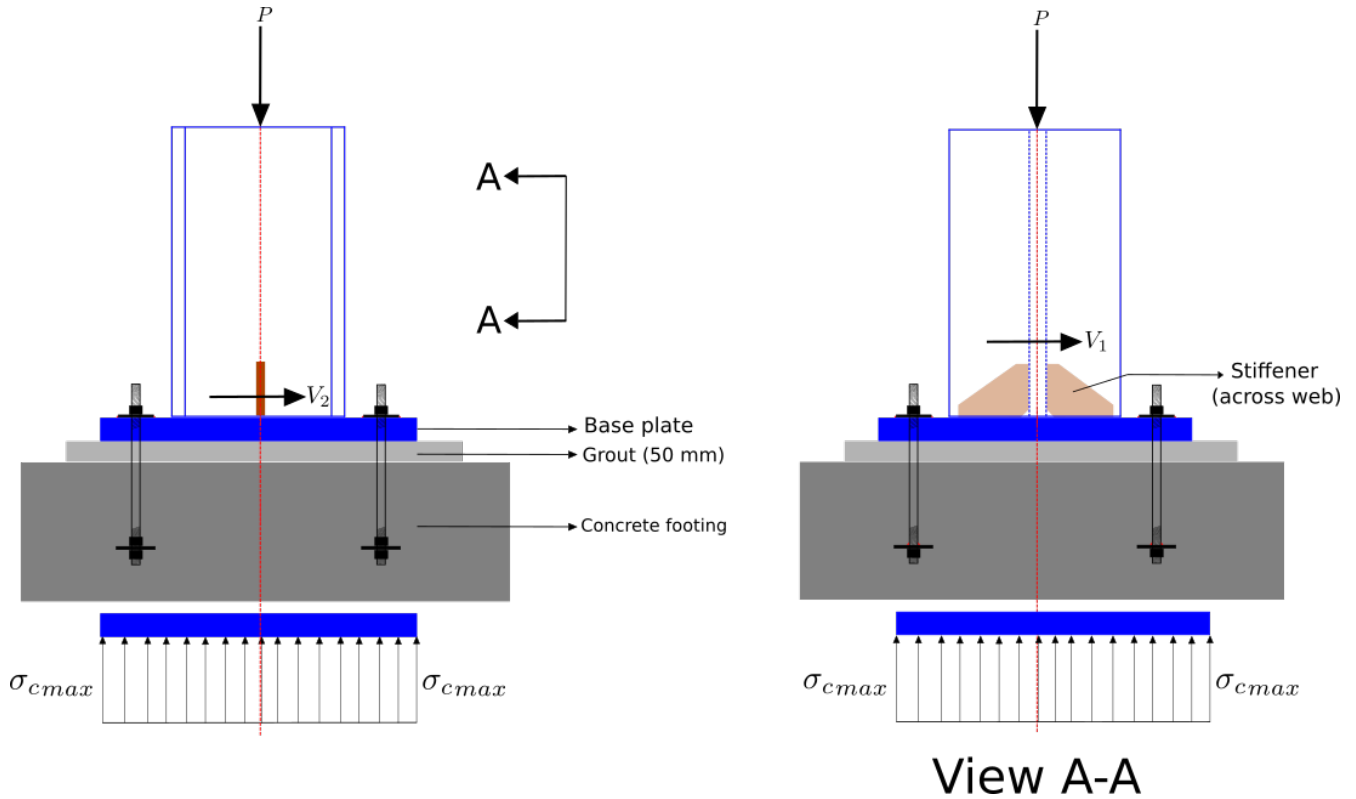


Figure 1: Typical Base Plate Details

3 2D Drawings (Typical)



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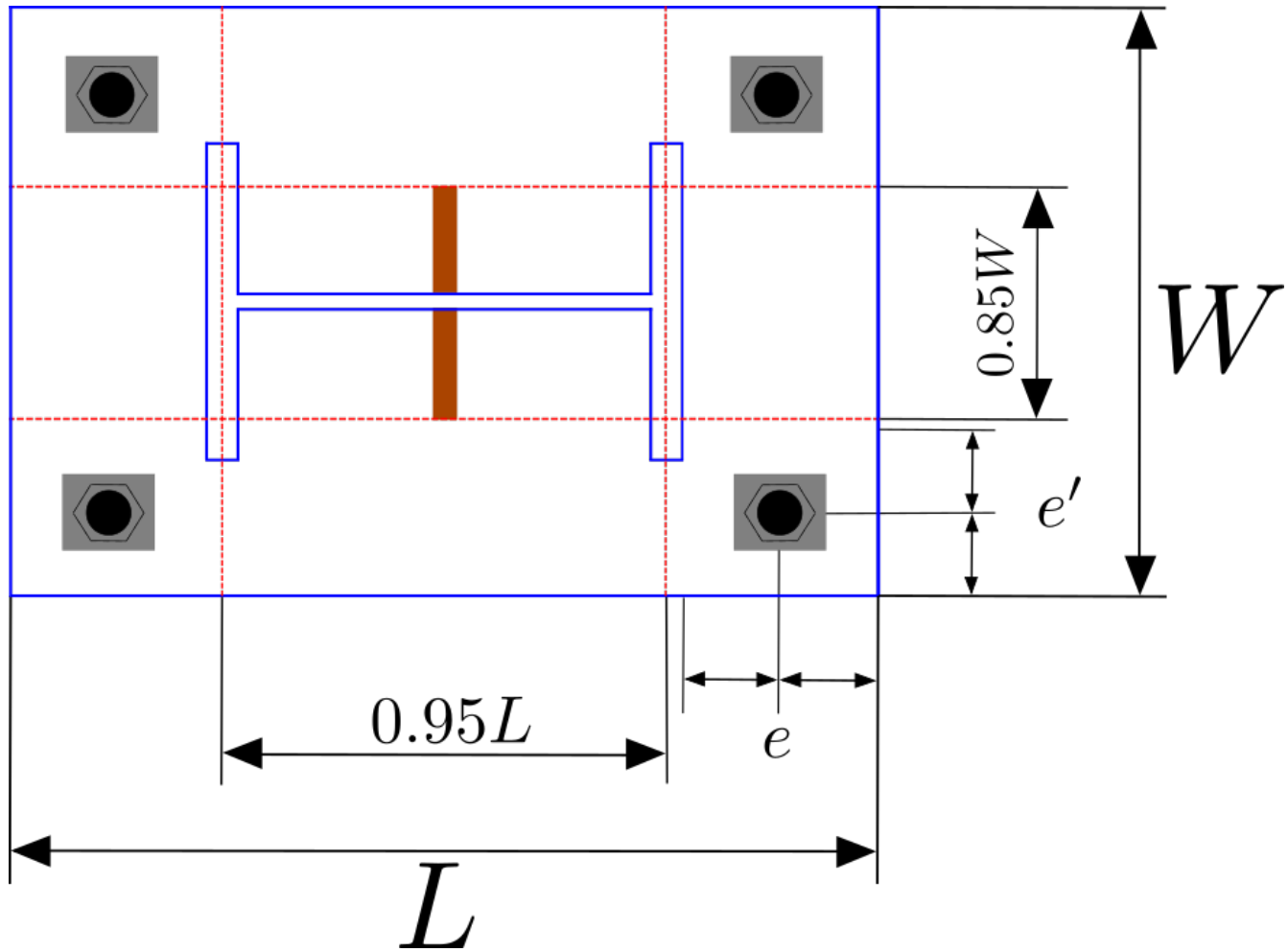


Figure 2: Typical Base Plate Detailing



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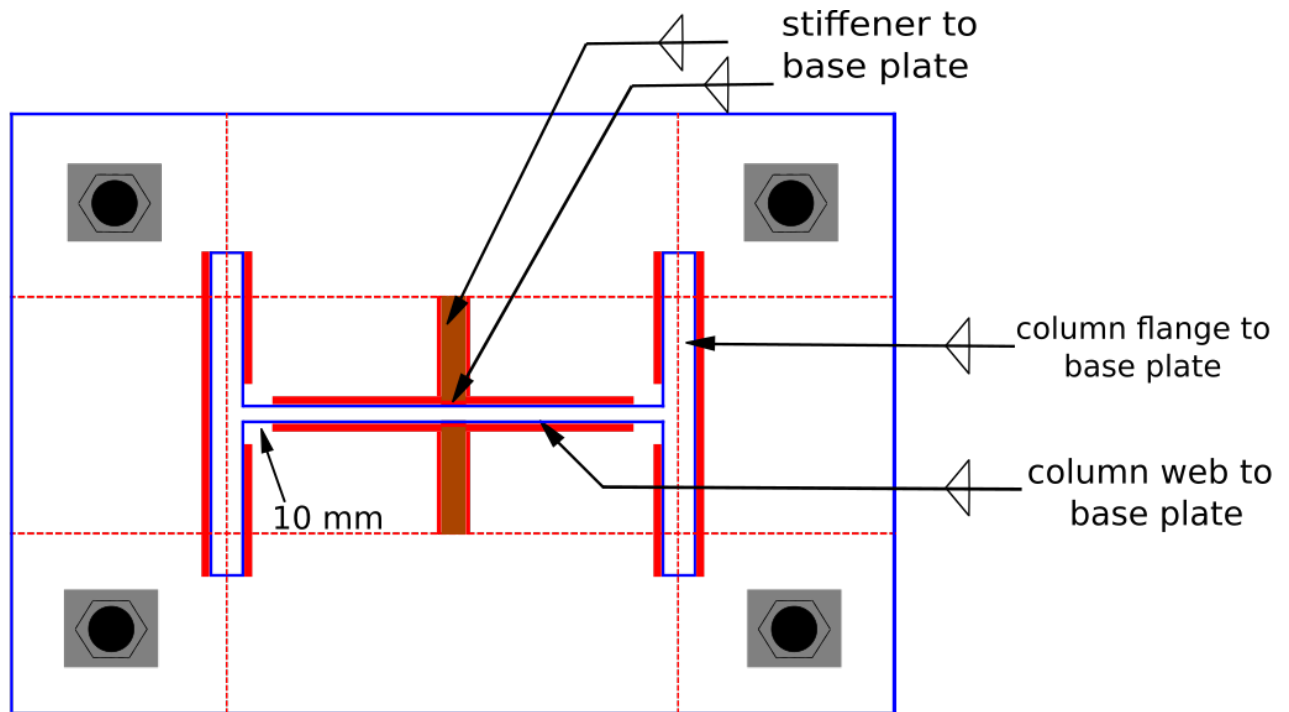
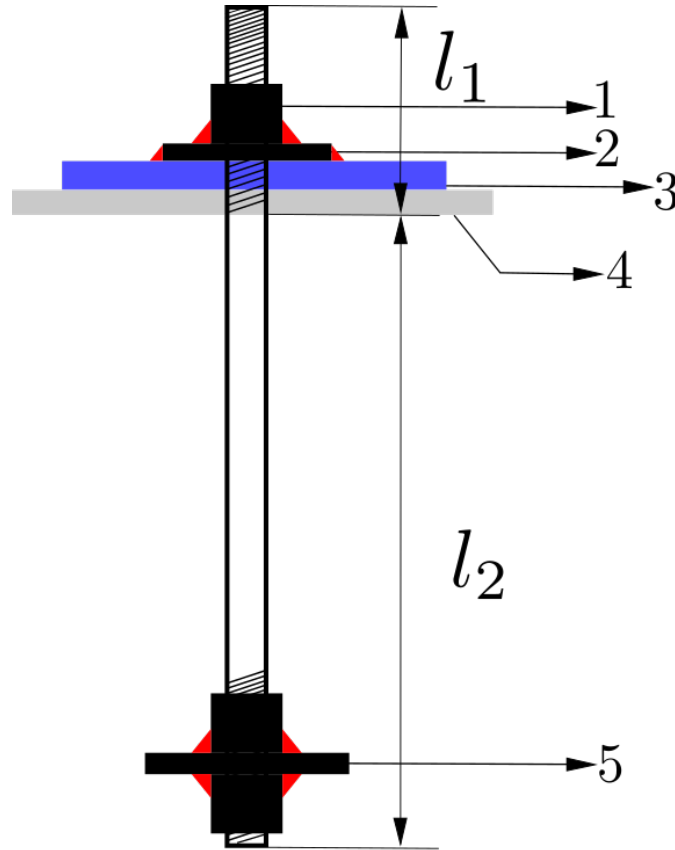


Figure 3: Typical Weld Details



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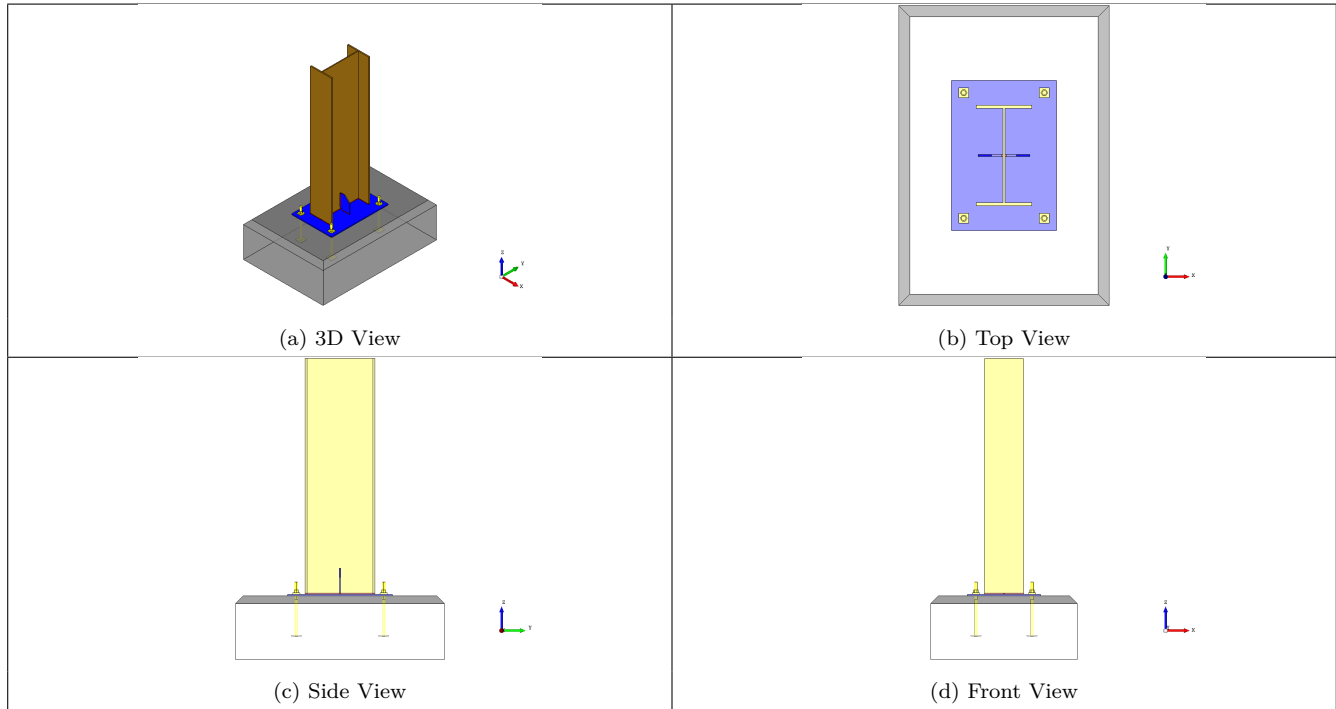
- l_1 = length above footing
 l_2 = length below footing
1 = t_n , nut thickness
2 = t_w , washer thickness
3 = t_p , plate thickness
4 = t_g , grout thickness
5 = end plate thickness

Figure 4: Typical Anchor Bolt Details



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4 3D Views



5 Design Log

2021-02-04 15:17:38 - Osdag - WARNING - : [Analysis Error] The value of the projection (c) as per the Effective Area Method is 15 mm [Reference: Clause 7.4.1.1, IS 800: 2007]

2021-02-04 15:17:38 - Osdag - WARNING - : [Analysis Error] The computed value of c should at least be equal to the end/edge distance

2021-02-04 15:17:38 - Osdag - INFO - : [Analysis Error] Setting the value of c equal to end/edge distance

2021-02-04 15:17:38 - Osdag - INFO - [Anchor Bolt Length] The recommended range for the length of the anchor bolt of thread size 20 mm is as follows:

2021-02-04 15:17:38 - Osdag - INFO - [Anchor Bolt Length] Minimum length = 200 mm, Maximum length = 800 mm.

2021-02-04 15:17:38 - Osdag - INFO - [Anchor Bolt Length] The provided length of the anchor bolt is 348.5 mm

2021-02-04 15:17:38 - Osdag - INFO - [Anchor Bolt] Designer/Erector should provide adequate anchorage depending on the availability of standard lengths and sizes, satisfying the recommended range

2021-02-04 15:17:38 - Osdag - INFO - [Anchor Bolt Length] Reference: IS 5624:1993, Table 1

2021-02-04 15:17:38 - Osdag - INFO - : ===== Design Status =====

2021-02-04 15:17:38 - Osdag - INFO - : Overall base plate connection design is SAFE

2021-02-04 15:17:38 - Osdag - INFO - : ===== End Of Design =====